INSPECTION REPORT STEAMSHIP PIER

NEW BEDFORD WATERFRONT FACILTIES INSPECTIONS NEW BEDFORD, MASSACHUSETTS



MARCH 2009



EXECUTIVE SUMMARY

Steamship Pier is comprised of three principal components: a timber boardwalk, the original pier structure, and the pier extension. The pier currently provides berthage for commercial fishing vessels.

Pare Corporation of Foxboro, Massachusetts, conducted the topside inspection of the site on November 14, 2008. The underwater inspection was conducted on November 14, 2008 by Childs Engineering Corporation of Medfield, Massachusetts. In general, Steamship Pier was found to be in **Fair to Good** condition. Concerns and deficiencies at the site include deteriorated concrete pile jackets, loss of exposed timber pile section, missing and loose pile cross bracing, deteriorated concrete deck, corroded low water connection hardware, and damage to the fender piles, wale, and chocks.

High priority repairs include the encasement of deteriorated piles, reconnection of cross bracing, replacement of broken or excessively damaged fender components, and patching of the concrete deck. The opinion of probable cost for this work is in the order of \$579,600.

Lower priority repairs include the addition of a cathodic protection system, which will increase the remaining useful life of the facility. The opinion of probable cost for this work is in the order of \$105,360.

It is recommended that the facility be inspected at 3 to 5 year intervals to monitor deterioration of the facility components.



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SECTION 1 INTRODUCTION

1.1 – Background and Objectives

The New Bedford Harbor Development Commission (HDC) has retained Pare Corporation (PARE) and Childs Engineering Corporation (CEC) to perform an above and below water inspection, and to develop a report of existing conditions for the Steamship Pier in New Bedford, Massachusetts. Inspections were performed in accordance with the ASCE Manuals and Reports on Engineering Practice No. 101 – Underwater investigations: Standard Practice Manual. The major objectives of the inspection report are to provide the HDC with an assessment of existing conditions at the facility, and to substantiate requests for funding for the maintenance and repair of the facility.

1.2 – Scope of Work

The scope of this investigation is to provide an inspection and evaluation of the present condition of the pier and appurtenant structures, and to provide information that will assist in both prioritizing repair needs and planning/conducting maintenance and operation.

The investigation is divided into three parts: 1) provide a description of the facility, including review of available reports, investigations, and data previously submitted to the owner pertaining to the wharf and appurtenant structures; 2) perform a visual inspection of the site above and below water; 3) prepare and submit a final report presenting the evaluation of the structure, including recommendations for remedial actions, and associated costs.

SECTION 2 DESCRIPTION OF SITE

2.1 - Site Location

Steamship Pier is an approximate 675 foot long pier located within Bristol County in the City of New Bedford, Massachusetts. It is bordered by State Pier to its north and Coal Pocket Pier to the south. Steamship Pier is approximately 1.5 miles south of Route 195 along the New Bedford waterfront on Buzzard's Bay, south of State Pier and north of Coal Pocket Pier as shown in Figure 1 – Locus Plan.

2.2 - Facility Description

Steamship Pier was originally constructed in 1838. Currently listed on the National Register of Historic Places, it was formerly used as a ferry terminal for passengers looking to travel between New Bedford, Martha's Vineyard, Nantucket, Boston, New York City, and New London, CT. Steamship Pier is now primarily used for the berthing of commercial fishing vessels. It is comprised of three structures; an original pier, a pier extension constructed in 1986, and a timber boardwalk between Steamship pier and State Pier. The original structure was a 291-foot long timber pile supported pier, with timber stringers and decking. In 1977, reconstruction of the



original structure included encasing the original timber piles in reinforced concrete, construction of concrete stringers, and installation of a concrete deck. In 1986 the pier was extended 384 feet with a concrete deck supported by steel piles. The timber wharf is supported by timber piles and measures approximately 30 feet by 150 feet.

Existing drawings pertaining to the facility were located in the HDC Office. Prior inspection reports have not been located at the time of this writing.

SECTION 3 EXISTING CONDITIONS

3.1 – General

A visual inspection of the above water structures at Steamship Pier was performed by PARE Corporation on November 14 and 20, 2008. The topside inspections included a visual inspection of the timber Steamship Pier boardwalk, the original pier structure, the pier extension, and all fender systems at the pier. The inspection of the underside included a visual inspection of the piles and pile caps above the waterline as well as the underside of the deck along the Steamship Pier boardwalk, original pier structure, and pier extension. Photographs taken to document conditions observed during the inspections are available in Appendix A – Photographs. The underwater inspection of Steamship Pier was performed by Childs Engineering Corporation of Medfield, Massachusetts on November 14, 2008 in compliance with the ASCE *Underwater Investigations: Standard Practice Manual* (2001).

For reference purposes, a baseline was established along the waterside face of the timber wharf and concrete pier during the inspection. Station 0+00 was located at the northeast corner of the timber wharf and extended to station 15+72 at the southwestern corner of the pier. Observations were made in relation to their location along the baseline as appropriate and as noted herein. A numbering system created during the underwater pile inspection is shown on Figure 2 – Site Plan. The numbering for the original pier structure begins with Bent No. 1 at the intersection with the Steamship Extension and finishes with Bent No. 36 at the final inshore pile bent. The Steamship Extension numbering begins with Bent No. 1 at the intersection with the original structure and continues to Bent No. 17 at the most outshore pile bent.

3.2 Original Structure

The original structure measures approximately 291 feet long by 61 feet wide. Thirty-six pile bents contain between 8 and 10 piles each, with a total of 343 piles as shown in Figure 2 – Existing Pile Plan. The original piles are timber with a minimum 2-foot embedment into a reinforced concrete section. The timber piles have been jacketed with reinforced concrete to increase capacity and protect the timber, which is understood to have been reduced by rot and marine borer. An 18-inch by 15-inch reinforced concrete pile cap supports 10-inch thick concrete deck slabs. A 20-inch high by 16-inch wide concrete curb is provided along the perimeter of the pier. Cleats are bolted to the top of the timber fender system wale at approximate 25 foot intervals over the length of the pier.



The original pier structure and pier extension are protected with a timber fender system comprised of 10-inch diameter piles spaced 8-feet on center, 8-inch by 8-inch timber chocks at the tops of the piles, and two 2-inch by 12-inch timber caps. An 8-inch by 12-inch timber upper wale runs continuous between the chocks and the concrete curb. An 8-inch by 8-inch timber lower wale runs continuous, bolted to the piles at Mean Sea Level.

3.2.1 Original Pier - Topside

The concrete deck of the original structure was observed to be in overall fair condition, with observed deficiencies consisting of stress cracks, deteriorating concrete, and exposed steel reinforcement. Typically along the length of the original structure, both transverse and longitudinal cracks were observed in the concrete deck as shown in Photo 3 in Appendix A - Photographs. Longitudinal cracks were spaced 4 feet on center, with transverse cracks were spaced approximately 24 to 25 feet on center, corresponding to the precast plank deck joints. The sealer in the expansion joints is typically loose or missing, with soil and gravel now filling the joints.

3.2.2 Original Pier - Underwater

The original section of Steamship Pier was reconstructed circa 1977 to include a precast concrete deck and reinforced concrete jackets around the existing timber piles. The supporting piles of the original section were observed to be in generally fair to poor condition. A number of the concrete jackets are disintegrating. (According to available drawings, these jackets form the connection between the timber pile and the concrete pile caps, and the concrete jackets actually form a concrete column above the top of the timber pile and below the concrete cap). Several of the jackets (ten) have completely broken down and only have the steel reinforcing cage remaining leaving the timber pile exposed. Other jackets have concrete section loss which exposes the rebar, both above and below the waterline. Some of the timber piles are exposed from one to four feet beneath the concrete jacket, and exhibit evidence of marine borer attack. Several piles have reduced pile diameters in this area, with the majority of the piles observed to display solid timber. Appendix A – Photographs indicates typical conditions encountered.

3.3 Pier Extension

The pier extension has added an additional 384-feet to the end of the original structure. The extension is approximately 38-feet wide for 288 feet. It then widens to approximately 63.5-feet wide for the remaining 96 feet. Seventeen pile bents are spaced 24-feet on center with 99 vertical piles and 32 batter piles. The piles are 16-inch diameter pipe piles filled with concrete. The pile caps are 32-inches wide by 30-inches deep and support 12-inch thick concrete deck slabs with a concrete curb that matches the curb on the original structure.



3.3.1 Pier Extension - Topside

Along the pier extension, the concrete deck was observed to be in generally fair to poor condition with several surface cracks observed. Several locations along the deck surface have spalled exposing the steel reinforcement, with significant spalling typically observed surrounding the expansion joints. Asphalt patching used during previous repairs was observed, typically with additional spalling of the concrete surrounding the patchwork as shown in Photos 9 and 10 in Appendix A. Spalling was observed at the edge of the deck from Station 6+56 to 6+62, Station 7+10, and Station 7+38. Specific locations of large spalls and other damage are listed in Appendix F – Field Notes.

The Steamship Pier extension was constructed around 1986 with precast concrete deck panels, concrete pile caps, and steel pipe piles filled with concrete. The extension is in overall good condition. The steel piles have significant coating remaining. There has been no significant loss of steel section. The underside of the concrete deck was observed to be in generally good condition. The concrete caps were observed to be in generally satisfactory condition, with typical areas of spalling and efflorescence noted along the caps.

3.3.2 Pier Extension - Underwater

The underwater inspection of the Pier Extension piles indicated that the steel piles are in generally good condition with no significant loss of steel section. Ultrasonic thickness (UT) and cathodic potential (CP) reading were taken along the steel pipe piles and steel sheet pile bulkhead. Readings were taken at the mudline, mean low water, and approximately halfway between the two. The following tables illustrate the results.

Table 3.2 – Steel Pipe Piles Underwater Readings

STATION	ELEVATION	UT Q1	UT Q2	UT Q3	UT Q4	СР
DENT 40	Mud	0.360	0.330	0.330	0.335	0.490
BENT 13 PILE 1	Mid	0.370	0.370	0.365	0.365	0.488
PILE I	MLW	0.395	0.365	0.375	0.370	.175430
DENT 44	Mud	0.320	0.325	0.330	0.330	0.507
BENT 11 PILE 1	Mid	0.370	0.390	0.360	0.370	0.502
PILE I	MLW	0.370	0.370	0.370	0.360	0.516
BENT 13.5	Mud	0.305	0.320	0.340	0.325	0.458
PILE1	Mid	0.370	0.355	0.360	0.365	0.448
PILEI	MLW	0.370	0.370	0.365	0.365	0.449
DENT 40	Mud	0.320	0.330	0.325	0.320	0.446
BENT 16 PILE 1	Mid	0.370	0.360	0.350	0.360	0.445
FILE I	MLW	0.370	0.370	0.370	0.370	0.454
DENT 0	Mud	0.325	0.330	0.315	0.330	0.511
BENT 9 PILE 1	Mid	0.350	0.370	0.360	0.370	0.510
FILE I	MLW	0.335	0.380	0.345	0.365	0.500
BENT 7	Mud	0.345	0.345	0.360	0.365	0.508



PILE 7	Mid MLW	0.370 0.365	0.375 0.365	0.375 0.370	0.380 0.380	0.508 0.510
BENT 5	Mud	0.335	0.340	0.345	0.330	0.513
PILE 1	Mid	0.365	0.385	0.380	0.375	0.514
PILE I	MLW	0.380	0.390	0.370	0.380	0.565
BENT 3 PILE 1	Mud	0.350	0.345	0.345	0.345	0.352
	Mid	0.365	0.365	0.365	0.365	0.324
FILE I	MLW	0.360	0.360	0.360	0.365	0.351
BENT 1 PILE 1	Mud	0.310	0.310	0.300	0.315	0.340
	Mid	0.380	0.375	0.380	0.380	0.349
	MLW	0.375	0.375	0.375	0.375	0.300

3.4 Timber Boardwalk

The timber boardwalk component of the structure runs parallel to the shoreline between State Pier and Steamship Pier. The wharf is supported by 12-inch diameter timber piles, 12-inch by 12-inch timber pile caps, and 4-inch by 8-inch timber stringers. The landside end of the pile caps is supported by a 16-inch by 4.5-inch by 0.75-inch double channel steel wale mounted to the face of a steel sheetpile bulkhead. The steel sheet bulkhead is retained by two inch diameter tie backs at approximate 10 foot intervals. The deck is comprised of 1-inch by 6-inch timber boards with a 12-inch by 12-inch timber curb along the waterside edge.

3.4.1 Boardwalk - Topside

The timber pile supported wharf extends from Station 0+00 to Station 1+54. The timber decking was observed to be in overall good to fair condition. Mildew growth was typical along the topside of the deck with some splintering and warping of the boards noted during the inspection. The pile caps extended approximately 12 inches beyond the waterside face of the wharf, with many of the ends damaged due to rot and possible impact from vessels. The timber curb was observed to be in fair condition, with rotting of the timber observed near Station 0+75, minor wear along the length, and a displaced section from Station 1+24 to Station 1+54. Four cleats were observed bolted to the top of the curb, however, two of them were damaged with bent bolts and broken timber at Station 0+94 and Station 1+09. A steel ladder that allowed access to a small floating dock was noted at Station 0+78. The ladder was observed to be in good to fair condition but was missing the connecting bolts and was held in place on the face of the wharf with a galvanized chain.

Beneath the timber wharf, the shoreline is retained by a steel sheetpile bulkhead. The bulkhead was observed to be in overall fair condition, with corroded steel and loss of the steel coating. A double channel steel wale runs along the face of the bulkhead and supports the timber pile caps at their landside end. The steel wale was observed to be in poor condition, with significant corrosion and 100% section loss observed in several locations.



3.4.2 Boardwalk - Underwater

The Steamship Boardwalk is supported timber pile bents comprised of 3 evenly spaced timber piles. Atop the piles is a timber cap that is supported at the inshore edge by the steel bulkhead wale. The timber piles are in generally good condition, with a few outshore piles exhibiting section loss due to abrasion from floating docks which are tied alongside. The out-shore end of the pile caps are typically rotted and split starting between pile 2 and pile 3 to the end which is cantilevered 1 to 2 feet beyond pile 3. The timber bracing is in good condition; however the lower connection hardware has typically 50% loss of the steel on the lower connections.

The steel sheet pile bulkhead is in fair condition. There is typically a ¼ inch of corrosion byproduct build up, pitting of the steel up to 1/8 inch deep and ½ inch wide, and low potential readings around 0.3 volts indicating a high corrosion potential. The steel sheet pile section located at station 0+00 has very thin steel readings at the mudline with less than a ¼ inch of steel thickness found in this area. It was noted that the sheet pile sections at this location are a different steel section than the remaining bulkhead section. The wall has an external wale with 2" diameter tierods spaced at 10ft intervals. The wale also supports a 12x12 timber pile cap which is the inshore support of the timber pier. There is an outfall in the steel sheet pile bulkhead located between bents 10 and 11 of the steamship boardwalk. The bulkhead runs out along the outfall in this location and is supported by steel struts 6 feet on center. The steel struts are in good condition. The bulkhead that flanks the outfall acts as the pier support for bents 10 and 11. The 12x12 timber cap is set along the top of the sheeting and supports the stringers between these bents.

Table 3.3 – Steel Sheet Pile Underwater Readings

STATION	ELEVATION	UT Inner Flange	UT Web	UT Outer Flange	СР
0+00	Mud	0.305	0.245	0.225	0.304
	Mid	n/a	n/a	n/a	no reading
	MLW	0.325	0.335	0.330	0.303
0+68	Mud	0.520	0.375	0.500	no reading
	Mid	0.515	0.365	0.505	no reading
	MLW	0.515	0.370	0.510	no reading

STATION	ELEVATION	UT Inner Flange	UT Web	UT Outer Flange	СР
0+00	Mud	0.305	0.245	0.225	0.304
Bulkhead					
Corner	Mid	n/a	n/a	n/a	
6' of Water	MLW	0.325	0.335	0.330	0.303
Bent 4	Mud	0.520	0.375	0.500	
0+68	Mid	0.515	0.365	0.505	



	MLW	0.515	0.370	
Bent 18	Mud	0.415	0.280	0.319
1+82	Mid	0.420	0.205	0.306
	MLW	0.520	0.355	

3.5 Fender System and Appurtenances

The fender system was observed to be in overall fair to poor condition. The 10-inch diameter fender piles were observed to be in good to fair condition, with several areas in poor condition. The timber chocks, upper wale, and fender cap were observed to be in overall fair to poor condition. Typical deficiencies consist of wearing of the pile faces and several missing and broken piles. Many of the timber chocks and several sections of the timber fender cap were broken or missing. Fire damage was observed on the fender cap at Station 4+94, creating a 12-inch diameter hole in the cap. Another fire has caused significant damage to the fender cap, upper wale, and chocks at Station 8+28. Typical fender system damage is shown in Photo 5 in Appendix A - Photographs. On the top of the upper wale, 24-inch long cleats were observed spaced 24 feet on center. It was noted during the inspection that numerous cleats were loose, with either bent connection bolts or damaged timber. Specific locations of broken and/or missing piles and chocks observed during the topside inspection are listed in the table below.

Table 3.1 – Fender Deficiencies

LocationStationDeficiencyDescriptionTopside2+31 - 2+39Timber ChockDamaged / BrokenTopside2+87 - 2+95Timber ChockMissingTopside3+03 - 3+11Timber ChockMissing	
Topside 2+87 - 2+95 Timber Chock Missing Topside 3+03 - 3+11 Timber Chock Missing	
Topside 3+03 - 3+11 Timber Chock Missing	
Topside 3+49 Timber Chock Significantly worn	
Topside 3+68 Timber Fender Pile Missing	
Topside 3+68 Timber Chock Broken	
Topside 4+17 - 4+25 Timber Chock Missing	
Topside 4+94 Timber Fender Pile Fire damage to top of	of pile
Topside 5+18 - 5+26 Timber Chock Missing	
Topside 6+22 - 6+30 Timber Chock Missing	
Topside 6+54 Timber Fender Pile Significantly worn	
Topside 8+03 Timber Chock Broken	
Topside 8+06 Timber Fender Pile Broken	
Topside 8+24 Timber Chock Missing	
Topside 8+28 Timber Chock Fire damage	
Topside 8+36 Timber Chock Missing	
Topside 8+41 - 8+49 Timber Chock Missing	
Topside 8+81 Timber Fender Pile (3) Significantly worr	า
Topside 9+76 Timber Fender Pile Broken at MLW	
Topside 9+76 Timber Chock Missing	
Topside 9+96 Timber Chock Missing	
Topside 10+60 Timber Fender Pile Significantly worn	
Topside 10+60 - 10+68 Timber Chock Missing	
Topside 11+41 Timber Chock Missing	
Topside 11+45 Timber Chock Missing	
Topside 12+41 Timber Chock Significantly worn	



Topside	12+98	Timber Chock	Missing
Topside	13+10	Timber Chock	Broken
Topside	13+87	Timber Chock	Broken / Significantly worn

Below water, the timber fender system is in fair condition. There are a number of broken fender piles and other broken timber members. Most of the piles show evidence of section loss due to abrasion. The timber members are solid below water with no observed punkiness.

The concrete curb houses a utility raceway that provides electricity to electrical outlets along the curb face and light posts mounted on top of the curb. Access boxes were typically located approximately 4 to 6 feet from each outlet and light post. From Station 5+42 to Station 5+58, spalling was observed on the face and on the top of the curb with no exposed reinforcement noted.

Galvanized steel ladders and life rings were observed along the original structure and pier extension. The ladders are mounted within the fender system and provide access to and from the deck and the water below. Overall the ladders were observed to be in fair to poor condition. Many of the ladders have been damaged during impact with vessels causing damage to the rungs and buckling of members. All of the ladders are corroded below mean high water rendering them useless during times of low water. The life rings were observed mounted to the face of the concrete curb, providing access to emergency life-saving devices.

A list of specific locations of all fenders, ladders, cleats, light posts, electrical outlets, and life rings as well as any other deficiencies observed during the topside inspection is provided in Appendix E: Field Notes.

SECTION 4 – STRUCTURAL CONDITION ASSESSMENT

4.0 – Structural Condition Assessment

4.1 General

Based on the observations obtained from the site inspections, the following provides our assessment of the various structures and components. Existing structure conditions were based on visual and tactile observations only, and were limited to accessible and visible portions of the structures.

4.2 Original Pier

The Original Pier component of the Steamship Pier was observed to be in generally fair condition, with observed deficiencies as described below:

 Cracking of the concrete deck is symmetrical with the precast concrete deck panels, and is typical of this type of construction. Lateral loads causing deflection of the pier



structure induce movement in the precast slabs, causing cracking of the concrete deck overlay.

- The concrete caps are in fair condition, with localized areas of repair required.
- The concrete encasements (jackets) on the timber piles were observed to have areas of deficiencies as indicated on Figure 2 Existing Site Plan. The loss of concrete section exposing reinforcing steel above the waterline must be repaired or replaced, as this area comprises a structural column and is required for the support of the pier. The loss of concrete cover on the underwater sections of the concrete encasements is not as critical, as this area of concrete serves primarily as a protection to the timber.
- Loss of timber pile section was observed in areas where the concrete encasements do not extend to the mudline. While this can be a potentially serious situation, the observed remaining pile diameters were generally good, with few exceptions, and solid timber was observed.
- Missing or disconnected pile bracing.

Given the age and existing condition of the pier, the original section of the Steamship Pier could be considered as nearing the end of its useful life. If neglected, the deterioration of the concrete encasements will result in a reduced load carrying capacity of the pier, requiring the eventual reconstruction of this component of the pier. However, with remedial work and continued monitoring, the remaining life of the structure can be extended.

Recommended remedial measures include the replacement of deteriorated concrete pile encasements, extension of concrete jackets to the mudline where pile diameters have been significantly reduced, and repair of limited cracking and spalling of the reinforced concrete pile caps. It is also recommended that the piles be inspected at regular intervals of approximately 3 years, to monitor further deterioration of the concrete encasements and deterioration of the unprotected areas of the timber piles.

4.3 Pier Extension

The steel pile supported Pier Extension was constructed in 1986 and is in generally good condition. The concrete deck was observed to have localized areas of spalling adjacent to the expansion joints which need to be addressed. The concrete pile caps were observed to have localized areas of spalling. The steel pipe piles were observed to be in generally good condition, with ultrasonic thickness readings on the steel pipe piles indicating very little section loss to date. Higher rates of corrosion were observed near the mudline. The installation of cathodic protection along the piles will reduce the rate of steel corrosion and therefore increase the life span of the structure.

Recommendations for this component of the structure include the repair of spalling and exposed reinforcing steel of the concrete deck and pile caps, and the installation of a cathodic protection system for the steel piles.



4.4 Timber Boardwalk

The existing timber boardwalk was observed to be in generally fair condition, with several areas of rehabilitation required. The timber piles were observed to be in generally good condition, with little evidence of marine borer. The existing lower bolts on the pile bracing are corroded and replacement is warranted. The timber pile caps are deteriorated above the outer row of piles, and need to be reinforced or replaced. The steel wale ledger beam along the inshore face of the sheeting is corroded and needs to be reinforced or replaced. The steel sheet piling is corroded and has a slight loss of steel section. Cathodic protection of the steel sheet piling is recommended to extend its useful life.

4.5 Fendering and Appurtenances

The existing fender system is in fair condition, with replacement of broken fender piles and wales and chocks required to adequately protect the structure and vessels. Similarly, the existing ladders should be repaired and replaced. The existing cleats have rotated in many instances and should be replaced with more substantial hardware fastened to the concrete.

SECTION 5 RECOMMENDATIONS AND OPINION OF PROBABLE COST

5.1 – Recommendations - General

Based on conditions observed during the inspections, and the corresponding assessments of the existing structures, the following recommendations are provided for the repair and rehabilitation of these structures. Existing structure conditions and assessments were based on visual and tactile observations only, and were limited to accessible and visible portions of the structures.

Opinions of probable cost were generated based upon current industry unit prices for similar work. Breakdowns of cost are provided in the Appendix. The opinions provided are for construction only and do not include allowances for engineering, permitting, or construction administration. A 20 percent contingency has been included with these costs. The opinions shown herein are based on a limited investigation and are provided for general information only. This should not be considered an engineer's estimate, as actual construction costs may be somewhat less or considerably more than indicated, due to fluctuations in the market and the actual repair implemented.

5.2 – High Priority

The following items are considered to have a High Priority, as they affect the usability and safety of the structure:



A. Rehabilitate Original Structure Piles

This item consists of the encasement of the severely deteriorated piles supporting the original structure of Steamship Pier. Providing "fish plates" or concrete jackets as means of repair is the most viable option at this site. Similar repairs to the piles beneath the pier were completed in the past.

The proposed work will require excavation to uncover the section of good pile below the mudline, and will include the encasement of the piles with reinforced concrete within fiberglass forms. The opinion of probable construction cost for this work is approximately \$278,000.

Alternatively, the replacement of the entire original structure with concrete filled pipe piles similar to the pier extension has an associated opinion of probable construction cost of approximately \$3,250,000.

B. Reattach Cross Bracing

This item consists of the removal and replacement of low water connection hardware for the cross bracing. Loose cross bracing beneath the original structure and timber boardwalk needs to be reattached. The opinion of probable construction cost for this work is approximately \$30,000.

C. Fender Repairs

This item consists of the removal of broken fendering members and replacement with new timber. This includes removal of old hardware, disposal of materials, and the furnishing and installation of new hardware and timber members. The opinion of probable construction cost for this work is approximately \$150,000.

D. Concrete Deck Repairs

This item consists of repairs to the deteriorated concrete deck along the pier extension. The proposed work will include patching spalled areas and installation of joint filler. The opinion of probable construction cost for this work is approximately \$25,000.

5.3 – Lower Priority

The following items are considered to have a Lower Priority, as they presently do not affect the usability and safety of the structure, but will need to be addressed in approximately 5 to 10 years.

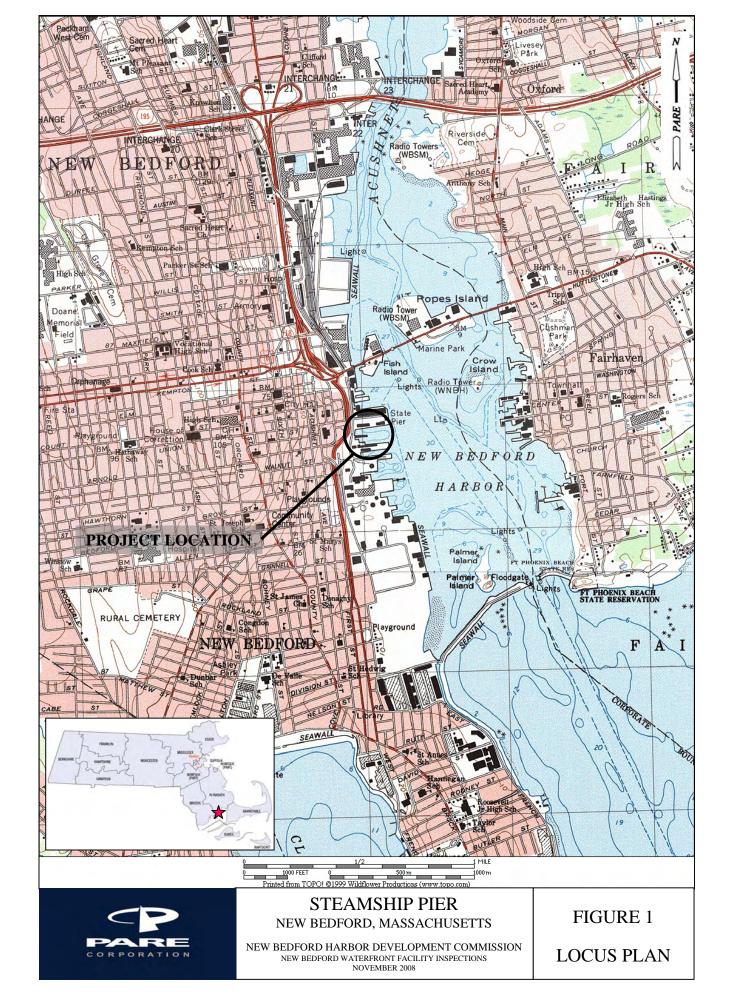
A. Installation of Cathodic Protection

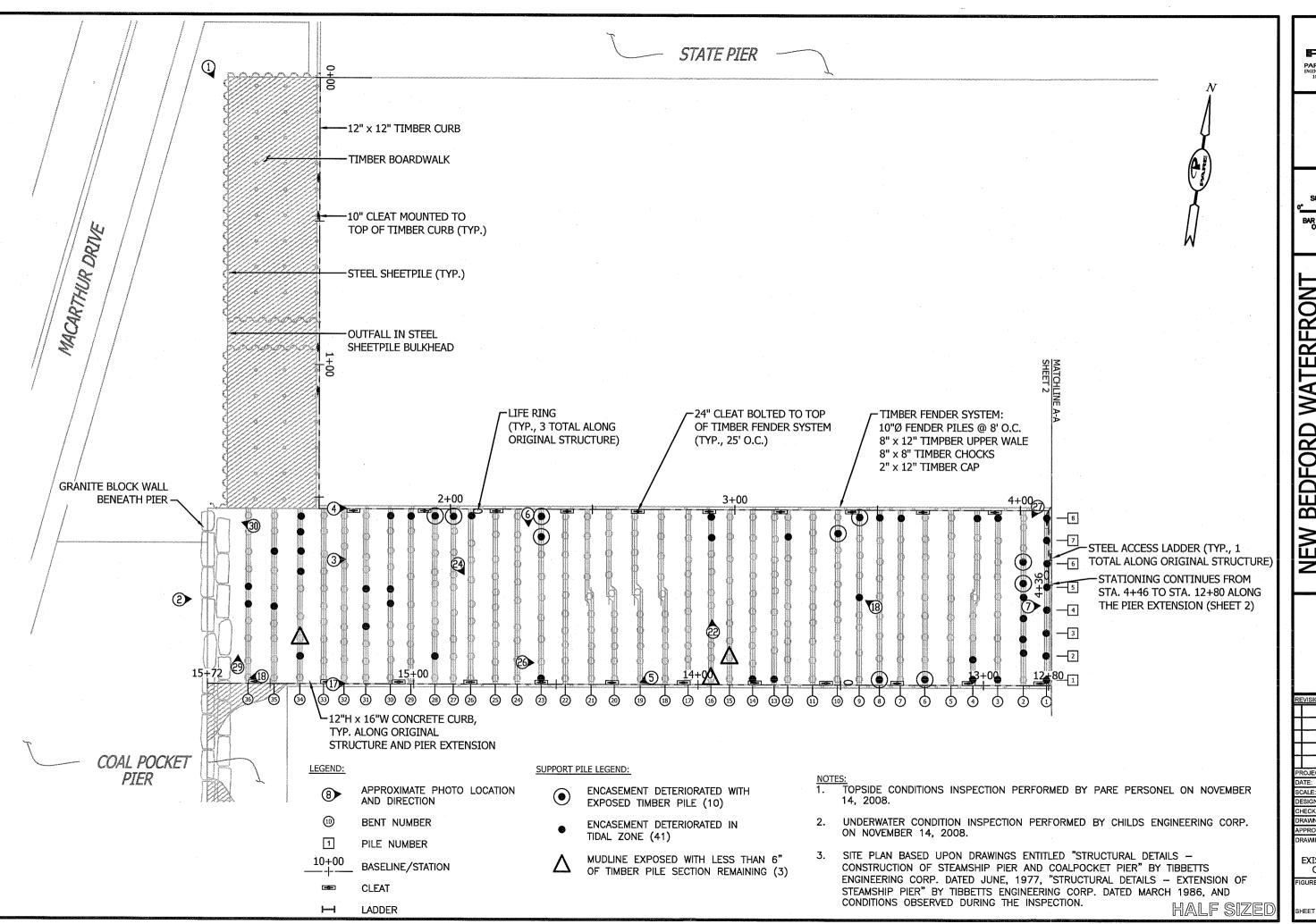


This item consists of the installation of cathodic protection along the steel sheetpile bulkhead beneath the timber boardwalk and on the steel pipe piles beneath the pier extension. Sacrificial zinc or aluminum anodes minimize corrosion of the steel sheet piling and components, as well as the steel pipe piles, and will extend the useful life of the structures. The opinion of probable construction cost for this work is approximately \$105,360.



Figures Steamship Pier New Bedford, Massachusetts







PARE CORPORATION
ENGINEERS - SCIENTISTS - PLANNERS
10 LINCOLN ROAD, SLITE 103
FOXBORO, MA 02035
508-543-1755

SCALE ADJUSTMENT GUIDE

BAR IS TWO INCHES ON ORIGINAL DRAWING.

WATERFRONT ITIES INSPECTIONS STEAMSHIP PIER BEDFORD

08216.0 NOVEMBER 2008

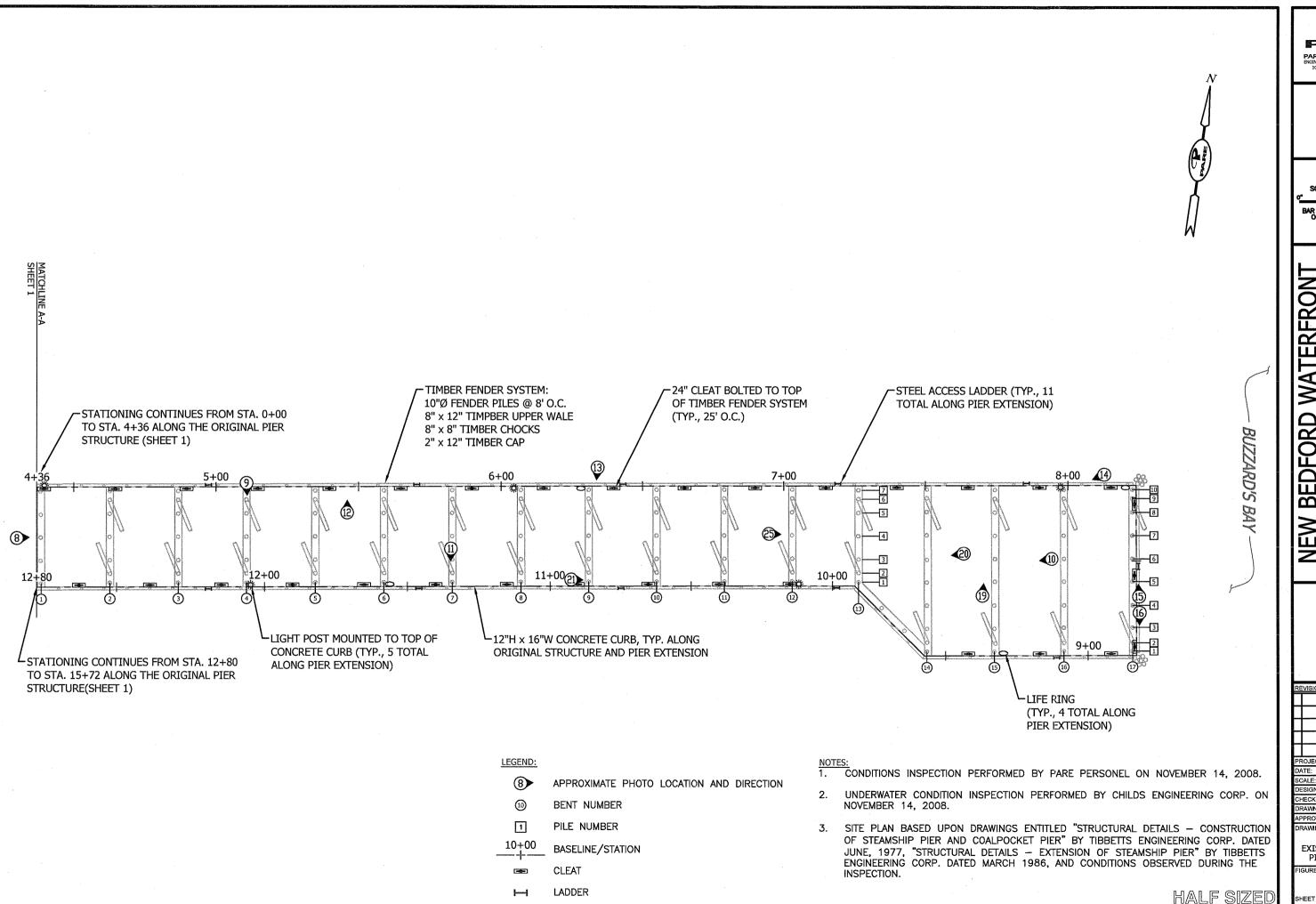
AS NOTED

SCALE: DESIGNED BY CHECKED BY:

RAWN BY: APPROVED BY

EXISTING SITE PLAN ORIGINAL PIER

SHEET NO. 1 of 2





PARE CORPORATION
ENGINERS - SCIENTISTS - PLANNERS
10 LINCOLN ROAD, SUITE 103
FOXBORO, MA 02035
508-543-1755

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-W BEDFORD WATERFRONT FACILITIES INSPECTIONS STEAMSHIP PIER

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RAWING TITLE:

EXISTING SITE PLAN PIER EXTENSION

2 of 2

Appendix A Photographs Steamship Pier New Bedford, Massachusetts



Photo No. 1: Timber wharf from Station 0+00 to Station 1+54.



Photo No. 2: Overview of the Steamship Pier.



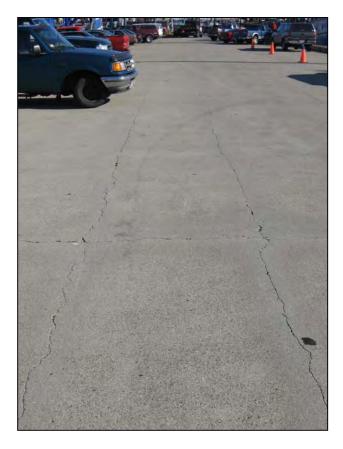


Photo No. 3: Typical longitudinal and transverse cracking observed along the deck of the original structure.

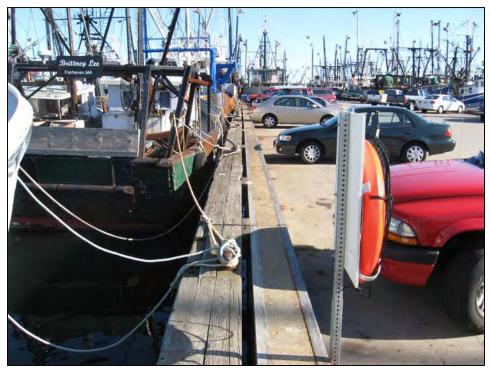


Photo No. 4: Overview of the concrete curb and fender system on the north face of the pier.



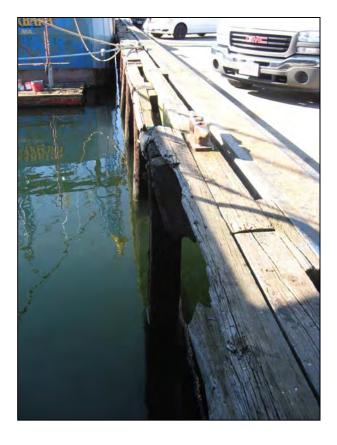


Photo No. 5: Typical fender section with broken chock and damaged or missing cap board.



Photo No. 6: Typcial expansion joint in the deck of the original structure. Note missing joint sealer.





Photo No. 7: Loose joint filler at intersection of original structure and pier extension.



Photo No. 8: Overview of the pier extension.





Photo No. 9: Typical expansion joint in the deck of the pier extension. Note spalling of concrete and asphalt patch.



Photo No. 10: Four way intersection of expansion joints at the head of the pier extension.





Photo No. 11: Spalling of the concrete deck with exposed reinforcement observed in several locations.





Photo No. 12: Spalling of concrete on top and face of curb.





Photo No. 13: Typical buckled ladder.



Photo No. 14: Broken fender pile at Station 8+06.





Photo No. 15: Damaged fender system from station 8+20 to Station 8+65.



Photo No. 16: Worn fender piles and loose cleat at Station 8+80.





Photo No. 17: Overview of the curb and fender system on the south face of the pier.



Photo No. 18: Patch in the southwest corner of the concrete deck.





Photo No. 19: Typical overview of the deck slabs, concrete pile caps, and piles below the pier extension.



Photo No. 20: Typical spalling of the pile cap beneath the pier extension. Note presence of efflorescence.





Photo No. 21: Significant spalling of the concrete pile cap along the waterside edge beneath the original pier structure. Note exposed reinforcement.



Photo No. 22: Typical spalling of the underside of the pile cap beneath the original pier structure.





Photo No. 23: Spalled areas on the concrete pile cap with rust staining from exposed steel reinforcement.



Photo No. 24: Cracking at the top of the concrete pile beneath the original pier structure.





Photo No. 25: Typical top of pile at the intersection of the concrete pile cap beneath the pier extension.



Photo No. 26: Missing concrete jacket with exposed steel reinforcement cage and timber pile. (Photo by Childs Engineering Corp.)





Photo No. 27: Missing concrete jacket with exposed steel reinforcement cage and timber pile.



Photo No. 28: Missing concrete jacket with exposed steel reinforcement cage in the tidal zone.





Photo No. 29: Overview of the granite block wall along the shoreline beneath the original structure looking north.



Photo No. 30: Overview of the void at the corner of the granite block wall and the steel sheetpile bulkhead under the timber boardwalk.



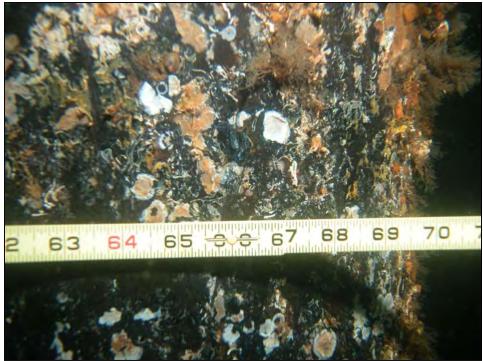


Photo No. 31: Typical marine growth below water. (Photo by Childs Engineering Corp.)

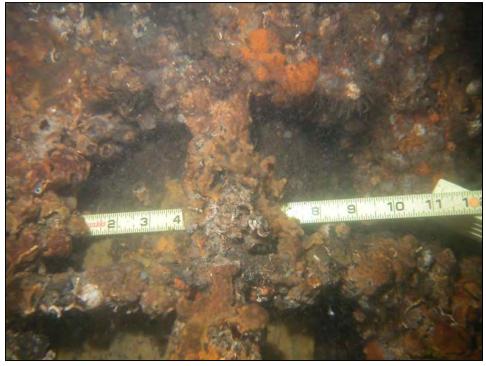


Photo No. 32: Underwater photo of exposed reinforcement. (Photo by Childs Engineering Corp.)



Inspection Date: November 14, 2008

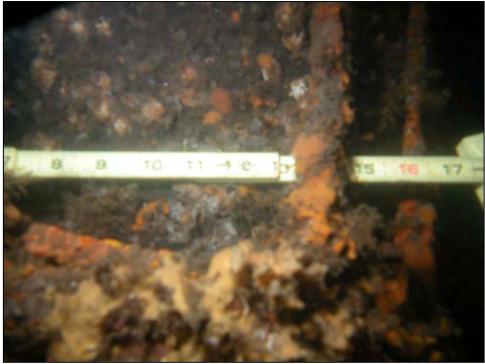


Photo No. 33: Typical view of the exposed steel reinforcement cage at the location of the missing concrete pile jacket. (Photo by Childs Engineering Corp.)



Photo No. 34: Underwater photo of marine growth on the pile. (Photo by Childs Engineering Corp.)



Appendix B Key Personnel Steamship Pier New Bedford, Massachusetts

KEY PERSONNEL

The following personnel were involved with this project including but not limited to the topside and underwater inspections and the preparation of this report:

Name	Employer	Responsibilities
Karl Hammond, P.E.	PARE Corporation	Project Manager, Lead Engineer
Ernest O. Rabideau, Jr., P.E.	PARE Corporation	Project Reviewer
Matt Bellisle, P.E.	PARE Corporation	Principal in Charge
Craig Sams, P.E.	Childs Engineering Corporation	Principal in Charge
Robert Garrity, P.E.	Childs Engineering Corporation	Project Engineer for Underwater Inspections
Kevin Champagne, P.E.	PARE Corporation	Support Engineer
Richard Fitzgerald, P.E.	Childs Engineering Corporation	Underwater Inspection Team
Charlie Marshall Roberts	Childs Engineering Corporation	Underwater Inspection Team
Robert Welch	Childs Engineering Corporation	Underwater Inspection Team
Phil Iantosca	Childs Engineering Corporation	Underwater Inspection Team
Nicholas B. Sarata	Childs Engineering Corporation	Underwater Inspection Team
Ryan McCoy	PARE Corporation	Topside Inspection
Briscoe B. Lang	PARE Corporation	Permitting Services



Appendix C Backup Data for Cost Estimates Steamship Pier New Bedford, Massachusetts

STEAMSHIP PIER OPINION OF PROBABLE CONSTRUCTION COST

February, 2009

	PIER REHAE	BILITATIO	N		
	QTY	UNIT	U	JNIT PRICE	TOTAL
High Priority Repairs					
1. Mobilization/Demobilization	1	LS	\$	10,000.00	\$ 10,000.00
2. Demolition and Removal	1	LS	\$	8,000.00	\$ 8,000.00
3. Pile Encasement	104	EACH	\$	2,500.00	\$ 260,000.00
4. Reattachment of Cross Bracing	1	LS	\$	30,000.00	\$ 30,000.00
5. Fender System Repairs	1	LS	\$	150,000.00	\$ 150,000.00
6. Concrete Deck Repairs	1	LS	\$	25,000.00	\$ 25,000.00
			Sub	total	\$ 483,000.00
			Con	tingency 20%	\$ 96,600.00
			Tota	al Alt.1	\$ 579,600.00
Low Priority Repairs					
Mobilization/Demobilization	1	LS	\$	10,000.00	\$ 10,000.00
2. Demolition and Removal	1	LS	\$	4,000.00	\$ 4,000.00
3. Install Cathodic Protection	6,150	LB	\$	12.00	\$ 73,800.00
			Sub	total	\$ 87,800.00
			Con	tingency 20%	\$ 17,560.00
			Tota	al Alt.2	\$ 105,360.00

PARE Project No.: 08216.00

Appendix D References Steamship Pier New Bedford, Massachusetts

REFERENCES

The following references were utilized during the preparation of this report and the development of the recommendations presented herein:

- 1. "About the Port Key Locations", New Bedford Harbor Development Commission, http://www.newbedford-ma.gov/PortofNewBedford/AboutPort/KeyLocations.html
- 2. Construction Drawings "Proposed Repairs and Improvements to Wharves and Piers in New Bedford and Fairhaven, MA", Tibbetts Engineering Corp., January 23, 1998 (Revised March 20, 1998).
- 3. Construction Drawings "Extension of Steamship Pier", Tibbetts Engineering Corp., March 1986.



Appendix E Field Notes Steamship Pier New Bedford, Massachusetts

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CHILDS ENGINEERING CORPORATION	PRE-DIVE MEETING & EQUIPMENT SAFETY CHECKLIST
Pre-Dive Meet	_ T
CEC Project #: Now Ked for I	ate :
Supervisor: Pf6 C	rew: RSW PD', RFG, NS RY
Dive Mode: Dive Plan Prepared: ☐ YES ☐ NO	Hazards Review
ØSSA □ SCUBA □ Wading	Water Current Maximum Depth
Objective of Dive:	✓ <1 Knot
Thermal Protection: V	☐ 1 to 2 Knots ☐ 10' to 20' ☐ 50' to 60'
☐ Wet Suit	☐ 2 to 3 Knots ☐ 20' to 30' ☐ 60' to 70'
Team Assignments Diver Repet G	—
Supervisor RFG Diver 1 PD 2500	
Tender Q V Diver 2 BW 2500	☐ Visibilityft ☐ Loss of Communication
Tender N Standby Diver	☐ Entrapment/Entanglement ☐ Underwater Debris
' /	☐ Break Away from Moored Station
Assessment of Team Members Physically Fit Ready to Dive	
Emergency Procedures	
Emergency Action Plan	Sonar Underway
☐ Emergency Numbers Available	☐ Tagout Notes:
Cell Phone/2-Way Radio Operational	Other:
Dive Team Attendees (Sign and Date) 1	P Un Date
2	Date Date
Pre-Dive Equipment	
Surface Supplied Air (\$SA) Fuel Compressor Oil Manifold Pressure Psi Repairs or Corrections Made:	Filters Engine Oil Connections
Roserve Air Supply:	0600 -
☐ HP Tank Pegulated to PSI Diver 1 Pony Tank ☐ HP Tank Regulated to PSI Diver 2 Pony Tank ☐ Standby Diver Pony Tank	PSI Tanks Secure PSI PSI PSI
Umbilical's, Whips and Pressure Gauges:-	
Connections Tight Main Valves Open Check for Main Blow Out Hoses Bleed H2O Out Of Volume Tank Repairs or Corrections Made:	nifold Leaks Check For Cuts in Umbilical's Non Return Valve
Dive Helmets:	
AGA Mask Nose Pad Swivel Connection Baild	out Block
SCUBA;	
Diver 1 Diver 2 Main Tank PSI Main Tank PSI	Sand-by Diver Pressure Gauge Main Tank PSI Depth Gauge
	Reserve Tank PSI SCUBA Masks
1	Standby Reserve Tank PSI Weight Belts
Primary Regulators Primary Regulators Backup Regulators Backup Regulators	Primary Regulators
Communications:	Dadway (Vogulators L) L AOA Wask
Diver Communication:	COMM SYSTEMS CHECK
Batterles Charged / Changed Surface Station Repairs or Cor rections Made:	L Test Diver 1 L Test Diver 2
General: Dive Logs Navy Dive Tables Navy	/ Repetitive Dive Classifications
Dive Supervisor :	
Signature	, Date

CORPORATIO			PRE-DIVE MEET	ING & EQUIPMENT
		Pre-Dive Meetir	g	
CEC Project # : Alton	1 th orangers	Wherf) Date	: 1119/19	
Supervisor: DDT		Crev	v: DUL 824	NBS RP"
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	•] YES ☐ NO		Hazards Review
☑ SSA ☐ SCUBA L Objective of Dive:	Wading		Water Current ⊠ <1 Knot	Maximum Depth ☐ <10' ☐ 40' to 50'
Thermal Protection:			1 to 2 Knots	
☐ Wet Suit ☑ Dry Suit	Motor Tomporet	ure <u>50 +</u> °f	2 to 3 Knots	1
	vvater i emperat	Diver Repet Gro		
Team Assignments	D:4	@ Start	>3 Knots	30' to 40'
Supervisor DDt	Diver 1		Loss of Air	⊠ Weather
Tender NB5	Diver 2		☑ Visibility ৡ ft	•
Tender KPN	Standby Diver よ	71)	-	ment 🖈 Underwater Debris
	F-70	P.D.	☐ Break Away from Mod	ored Station
Assessment of Team Members	Physically Fit	Ready to Dive	Ships at Berth	
	Emergency Diver Bailou			Props Rudder
		2/06	☐ Sonar	LJ Underway
	<u> </u>		Tagout Note	s:
Cell Phone/2-Way Radio Operation Dive Team Attendees (Sign and Date)			Other:	
Dive Team Attendees (Sign and Date)	4/.	1/04		
1 1 1	II/I	4_		Date
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	Pre	-Dive Equipment C		
Surface Supplied Air (SSA) Fuel Compressor Oil Manifold Pressure /_3 0 P Repairs or Corrections Made:	X Belts □x U si ✓ Volume T	Dive Compressor: Inloader 区 Fi ank		☑ Connections
Reserve Air Supply:				
☐ HP Tank ☐ HP Tank Regulated to		Diver 1 Pony Tank Diver 2 Pony Tank	PSI PSI	☐ Tanks Secure
		itandby Diver Pony Tank		
Umbilical's, Whips and Pressure Ga	uges:			
	Main Valves Open Bleed H2O Out Of Volu	L¥ Check for Manit me Tank		ck For Cuts in Umbilical's Return Valve
Dive Helmets:				· · · · · · · · · · · · · · · · · · ·
AGA Mask Nose Pad Repairs or Corrections Made:	Swivel Connection	on 🛚 🖾 Bailou	t Block	
SCUBA:				
Diver 1	Dive	r2	Sand-by Diver	Pressure Gauge
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	Reserve Tank		eserve Tank	PSI SCUBA Masks
Standby Main Tank PS Primary Regulators	Standby Reserve Tank Primary Regulators		andby Reserve Tank Imary Regulators	PSI Weight Belts BCD
Backup Reg ulators	Backup Regulators	·	ackup Regulators	☐ ☐ AGA Mask
Communications:				C.
Diver Communication: Diver Communication: Charged / Changed	Surface Surface		Test Diver 1	☐ Test Diver 2
Repairs or Corrections Made:	E Guitat	oo otanan	THE LOST DIVELLE	- Test Diver 2
General: N Dive Logs N Dive Flags Out N Ladder / E Repairs or Corrections Made:	Ⅸ Navy Dive Table ntry and Exist Points Secu		Repetitive Dive Classifications lotes	Dive Knife
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CHILDS ENGINEERING CORPORATION

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CHILDS ENGINEERING CORPORATION

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CHILDS ENGINEERING CORPORATION

Waterfront and Structural Engineering

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	P. LE 3 - HW to CW - Repar Exposed 2" Deep
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CHILDS ENGINEERING CORPORATION

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	β6	PIS P9 Transu Ex. PID-10"9 Ferder Pile Massing	10-4-1
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	85	DA	Concrete Tacket @ 1 Pretty much gove 3'-8' Relow CAP REBARIEM
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file 1	2 of tune exposed	14" Danete MB Evidence on 0575.	~ <i>]</i>			
		of Phe				
		Timber is solved				
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P./2:	3 Jacket No MUD					
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	timber PV/1	le exposed 12" Hard				
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CHILDS ENGINEERING CORPORATION

Waterfront and Structural Engineering BOX 333, MEDFIELD, MA 02052

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2" Coss Around Permiting of TACRUT STAILS Z'Below CAP Bent 12 Pile 8 RUNS to MUSLAC P1 - 4' Below CAD TO MUYS 1'DFLOSS (Ben7 13 - 9 Pins 707A) Bet 1 18 Bet 10 Bet 6' Aparl 13-14 Be 13-15- Con water whole Broken North Some TYPRAL 12" & Piles Ben 15 - 6" & P. le l'Exposel Belon Jacker, TOB PIE THE pa - JACKet Coss 2" Around Permut PTP10. John 3" COSA From CAP to Botton
Exposed Cellin 8 0 1 exposed Pile 1 1817

CHILDS CHILDS ENGINEERING CORPORATION

CHILDS ENGINEERING CORPORATION

Waterfront and Structural Engineering BOX 333, MEDFIELD, MA 02052

JOB New Bestard	
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Steam ship Pich OVD avue a site 0730 PPI RTW Oneis in @ 950 Ple 1 South Side Qual 3 POT Bent 29 Cracking top down 3' CCS - OCS And 2 yest in Fran Cap Pile cap in good condition Ple 3 Quad Z West of 3 G x & " u/ Rost stains Pile 1 - 1' of to be exposed Piles 2 x 3 just concrete Pile Cup 1'5 of Pile 4 17 5p W/ Pust Stein 1"deep Ple 41 Qued 142 vet Cr Z'x 18" must cracks 1' south of Pilo 6 - Pile cap QCs bottom & outshore face 3 A × Z'deep Ple & Qued Z-3 CC5 3 W Rust stains Rle 9 MLW down 4'-exposed agg, concrete Soft, 1-2 2'deep Quad 2 Must Cr. CC5 38 W/ Rust Simo @ top



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	130 t 22 R5W - bracing pre Ple 1 3 tuber exposed 13 Ple 9 3' " ">14'6 Ple 10 4'"	3" MB - 1"Hancles - Josef
	Bent ZI Pile 10 4 tember en Hos brung 9 2" No cop jog Pile 1 18 tember pile ex	pased 12" p mB minas
		ile 1-2 cornected, mat connected on piles 3, 4,5
	Bont 19 to be appeal 13 % go Pile is 7 pile appeal 13 % go Bont 18-20 ton walk on two Pile 8 6" sig exposed 12" go Pile 2 6" " " 8" & A	cool cond cool cond der misine cool cond hy MB 4" for

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		þ	them 4 of	pelet		
	Pile 6	5,0 batton	3' all grad	l'deep I Z'deep all Erom cap down		
	Pile 7	botton &	p RB expose	d zadiep all	grad	18 1 .
	Ple 10	Sp on ja	hot 12 d	eup WRB 5% re all grad	to 4 below	, cap
* y/w	PHOTO -	+ exposed (tinker 4'	MB15"-Z'd	rep poekë	4-
世				6"p	P /	
	13000	214 29 3	w/ cable.	expersed at	when 57	vate, broken
	Pheto 8	1 18-02	.1c 10 Ben	+ 33 - Morine	bore	
	Pile 10	Bent 34	bittom	ets agampate 1"	exposed	
		3	below cop	explagamale l"	- 3" decp	
		5	long section			
	P:12 9	bott.	, 2' of	pile exposed -	12" \$	
	2 63					
	Pik 8	qu. 3	11,1 expo	seed for conc. to, cop , 5' long	rs I deap	
		5401.	1 0c tow	Cop		
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Waterfront and Structural Engineering BOX 333, MEDFIELD, MA 02052

JOB	 	
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Bat 36 PDI 2 sq. Ft. 1" deep CC 5 Stemmish Between 4-7 each Section between piles all 3 Forces

Cracks 1/6" w/ keeching Bat 35 exbosed RB 1' From top of gic going 5'

Hyp. Liss 4" clock 8349 Steel builthood at N side of free - runs perportioner to pier us / make 3 ples running I to hallkhood

15t pik 12' From bourhold cop 5ts of web Steamskip BONEDWAIK Bont 17 Sta. 182 7 - D meter Midline outside 420" meb 450,280",319 pot reading Typ. steel incoated pits starting 1/8" deep mox 1/4" will a 1306 pat reading 355 web

.520 would f)

Steel smooth, 807, conting intent 285 205 420 Typ. steel 14 Hack bygoduct pils 1/9" dep 1/2" wide Oxtern wate - w/ 2" the rod w/ + 10 spaces 5ta 1+82 2 tre Rood

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BOX 333, MEDFIELD, MA 02052	CHECKED BY DATE
	SCALE STEAMSHID BOARDWAIK
19 timber piles outon of bullions timber pile cap site on bullions outstand 3 of pile cap best 2 pile back 1 from s	Aphend 10 Ben 18
12,14,15 Best 7,5 + 5 Same os Best	t Z on rele cap condition
lew water batts X bracing pile Batto @ Gru wo BFWT 15 Donater @ bert 9 5ta 0+68	le Z-3 + 1-2 + long. X brawies along pile 1, bost to best ster 50% corrolled
ant FI ,520") web 375 for ML &	Steel Jo70 contest
inclu f) 505	steelsmoth 1-pt & deep x 1918 Coale 50% whit
neb ,370" (8 197) =	So so intact coating Small pitting "& holes sheet - similar to earlier Steel bulkhead Fill make
In ber piles & brawis typ in low balts	

D 400 (X) Steamahil DXIV'S Grand Block wall 4+85 Homes

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			SCALE		
	PRITIZ			pile suppor	
agter	benton 1	$z \times 12$ on to	p of short	pile suppor	ting
		5-hzuzero	7 0		
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	Part 7 (ath cap re	the freezele	t outstare 5	12×17 -
		on shut s	Ye	toutelone 5	
6) 4	- Lullahe	1 James	Sta 0+00	6 of water	
1 mel	V & LOOK C				
act	· £1 ,305		- 4 "carrer	sin by produ	# 1
we	b ,245°	SmL 1	Continu	mosine	
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		Kood	"s old-steel	2-cooting 30	fr istart
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Tile to	my Bent 1 -	ontstone c	rated to	plit 3, browns 2	- 3
				and the state of t	
Ro	cap sits on	bulkhead	wale on mak	ne end	
Bent	8 Ple 3 -	abouted 3	0% from pi	le cap deum	5
	1 outs Q	ne pile	from Float	ing docle	
Berto				+0	. 2
10-9,	9-8 +8-7 M	o outstane a	veg bucins	on outshare p	Ull I I I I I I I I I I I I I I I I I I
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		SUALE	
	Bent 6 - rule cap ratted & C	rusped outstone 4	
	Bent 6 - pile cap ratted & c nat supported	by pile	
	bracing bolts 50% a		
	Bent 4 pile cap ratted 4 on	ttole and, 25% s	ection remains
	X-Brown Prem Borts 5404 9		
		c 241 258 A	879,400
	Bent 3 aut Rue 4' of pil Pile 3 druven et an angle	cap ramo, ester	
	Bent 2 Pile cap onto Dore 12	c notted - extends bey	and when face
1	Bert 1 Same as Bert 2		
	Dues ent 13/5		